

Designing Precast, Prestressed Concrete Bridge Girders for Lateral Stability

Richard Brice, PE

WSDOT Bridge and Structures Office

Introduction

- Lateral stability is a serious concern
- AASHTO LRFD BDS revised stability requirements (9th Edition, 2020)
- Designers should become familiar with stability requirements
- Overview of lateral stability theory
- WSDOT stability design practices

This is highly undesirable



Motivation

- Stability concerns are strongly emphasized by industry
 - PCI has published “*Recommended Practice for Lateral Stability of Precast, Prestressed Concrete Bridge Girders*”
- Updates to the AASHTO LRFD BDS, 9th Edition
- Current trends
 - Girder lengths are increasing (220+ feet)
 - Stability is problematic at modest span lengths for I-Beams (AASHTO Type I-IV)
 - Issues
 - SAFETY
 - stability failure has occurred
 - re-design after bidding because girders can’t be lifted or transported
 - contractors and fabricators don’t want to be EOR for the girders
 - change orders, additional reviews, increased cost, schedule implications

Engineering Roles

- Design Engineering for Service
 - Engineering performed by the engineer-of-record to ensure the in-service structure is safe, durable, and meets all design requirements (AASHTO, BDM, etc.)
- Design Engineering for Constructability
 - Built-in allowances provided in the design (explicitly or implicitly) to ensure a precast girder is able to be constructed with available means and methods at a reasonable cost. Typically performed by the engineer-of-record as needed to minimize changes under contract
- Construction Engineering
 - Engineering performed by the contractor to ensure actual construction means and methods do not damage the bridge or otherwise prevent the bridge from functioning as intended

Current LRFD Specifications (8th Edition)

Specified since 1st Edition 1994

1.3.1—General

Bridges shall be designed for specified limit states to achieve the objectives of constructability, safety, and serviceability, with due regard to issues of inspectability, economy, and aesthetics, as specified in Article 2.5.

2.5.1—Safety

The primary responsibility of the Engineer shall be providing for the safety of the public. The Owner may require a design objective other than structural survival for an extreme event.

2.5.3—Constructibility

Constructability issues should include, but not be limited to, consideration of deflection, strength of steel and concrete, and stability during critical stages of construction.

5.5.4.3—Stability

The structure as a whole and its components shall be designed to resist sliding, overturning, uplift and buckling. Effects of eccentricity of loads shall be considered in the analysis and design.

Buckling of precast members during handling, transportation, and erection shall be investigated.

Updated LRFD Specifications (9th Edition)

5.5.4.3—Stability

The structure as a whole and its components shall be designed to resist sliding, overturning, uplift and buckling. Effects of eccentricity of loads shall be considered in the analysis and design.

Buckling **and stability** of precast members during handling, transportation, and erection shall be investigated.

5.9.4.5 Temporary Strands

Temporary top strands may be used to alleviate tensile stresses in precast prestressed girders during handling and transportation. These strands may be pretensioned or post-tensioned prior to lifting the girder from the casting bed or post-tensioned immediately prior to transportation of the girder. Detensioning of temporary strands shall be shown in the construction sequence and typically occurs after the girders are securely braced and immediately before construction of intermediate diaphragms, if applicable.

C5.9.4.5 Temporary Strands

The stability of slender precast girders is improved when lifting and transportation support points are moved away from the ends of the girder. The consequence of having a shorter span between support points is reduced dead load stresses to balance the stresses due to pretensioning and thus excessive tensile stresses in the top flange and compressive stresses in the bottom flange may develop. Temporary strands placed in the top flange of the girder alleviate these excessive stresses and reduce the required strength at prestress transfer. Temporary strands in the top flange balance a portion of the primary prestressing and reduce camber and camber growth due to creep.

Updated LRFD Specifications (9th Edition)

5.12.3.2—Precast Beams

5.12.3.2.1—*Preservice Conditions*

The preservice conditions of prestressed girders for shipping and erection shall be the responsibility of the contractor.

Stability during handling, transportation, and erection can govern the design of precast, prestressed girders. Precast members should be designed such that safe storage, handling, and erection can be accomplished by the contractor using reasonable means and methods.

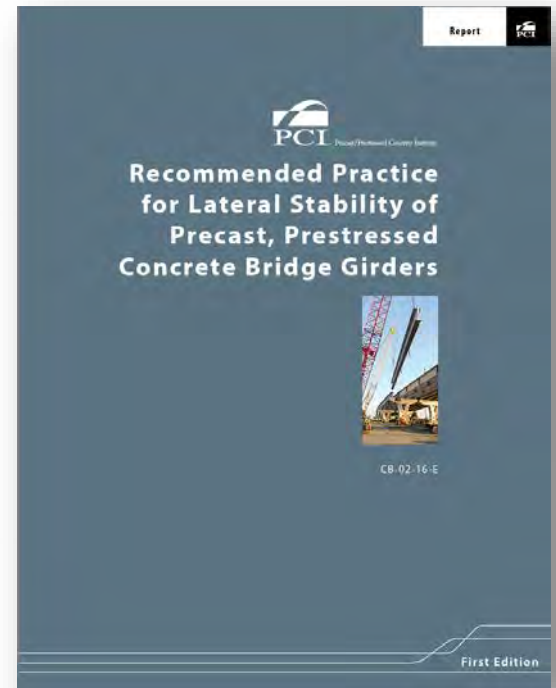
C5.12.3.2.1

AASHTO LRFD Bridge Construction Specifications place the responsibility on the Contractor to provide adequate devices and methods for the safe storage, handling, erection, and temporary bracing of precast members.

Lateral bending stability analysis may be based on the “Recommended Practice for Lateral Stability of Precast, Prestressed Concrete Bridge Girders”, Precast Concrete Institute, Publication CB-02-16-E. A detailed design example is presented in Seguirant, Brice, and Khaleghi, (2009).

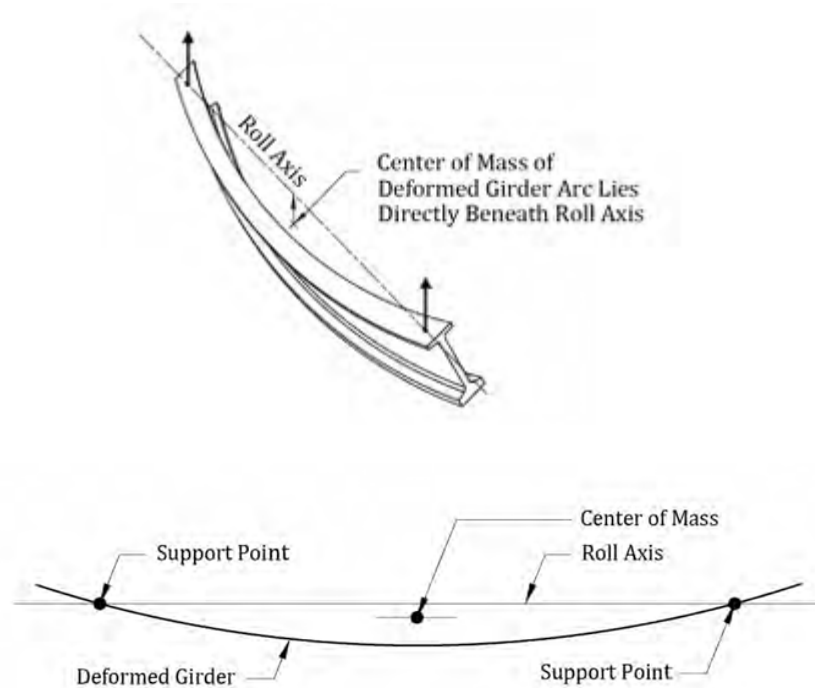
Stability analysis basics

- PCI – Recommended practices for lateral stability
 - Based on work of Robert Mast, PCI Journal Special Reports, Part 1 (1989), Part 2 (1993)
- Two primary cases of concern for bridge designers
 - Hanging girders (lifting)
 - Seated girders (hauling)
- Lateral **bending** stability
 - Girders assumed to be torsionally rigid
- Concerns
 - Statical equilibrium of the girder during handling
 - Ability of girder to resist lateral bending



Hanging girders

- Parameters: Girder stiffness, lift device rigidity, pick point location, pick eccentricity, camber, sweep



Stress Analysis

- Equilibrium

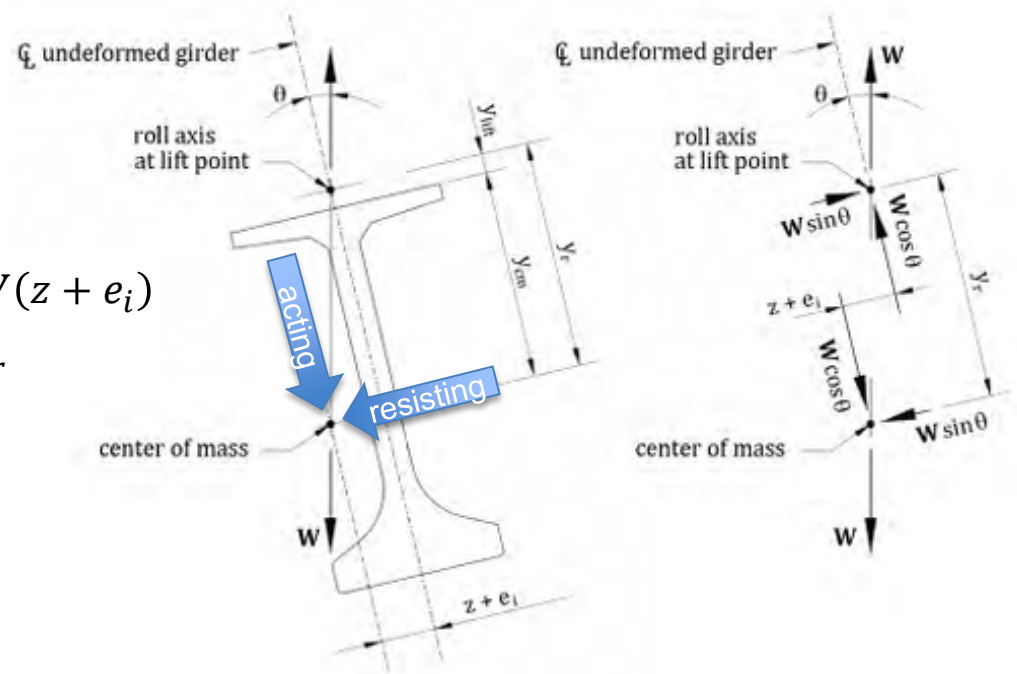
- $M_{acting} = M_{resisting}$
- $M_{acting} = W \cos(\theta)(z + e_i) = W(z + e_i)$
- $M_{resisting} = W \sin(\theta)y_r = W\theta y_r$
- Solve for θ

- Biaxial stresses

- $$f = \frac{P}{A} + \frac{(Pe+M)y}{I_x} + \frac{\theta Mx}{I_y}$$

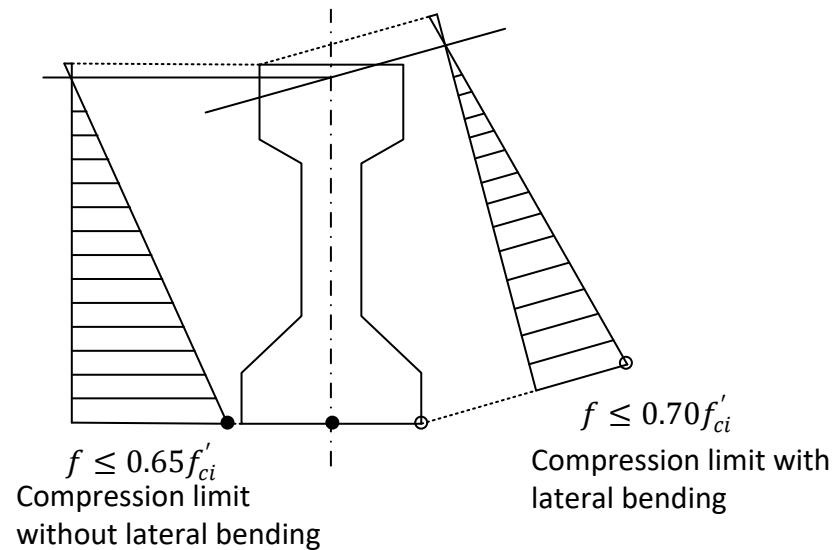
- Lateral offset of center of mass, $z + e_i$

- THIS IS NOT THE DEFLECTION OF THE CENTERLINE GIRDER
- z computed as offset that would occur if 100% of the self-weight is applied laterally (z_o) times the tilt angle
- $z = z_o\theta$
- $M_{acting} = W(z_o\theta + e_i)$



Stress Limits

- Ensure stresses are within acceptable limits
 - Tension: $0.0948\lambda\sqrt{f'_{ci}} \leq 0.200 \text{ ksi}$
 $0.24\lambda\sqrt{f'_{ci}}$ (with bonded reinforcement)
 - Compression: $0.65f'_{ci}$ (plumb) $0.70f'_{ci}$ (tilted)



Cracking

- At equilibrium, how close is the girder to cracking?
- M_{cr} cracking moment is the moment which gets $f = f_r$
- Compute tilt angle at cracking $\theta_{max} = \frac{M_{cr}}{M_x}$
- Compute acting and resisting moments at tilt which induces cracking
 - $M_{acting} = W(z_o\theta_{max} + e_i)$
 - $M_{resisting} = W\theta_{max}y_r$
 - $FS = \frac{M_{resisting}}{M_{acting}} \geq 1.0$

Failure

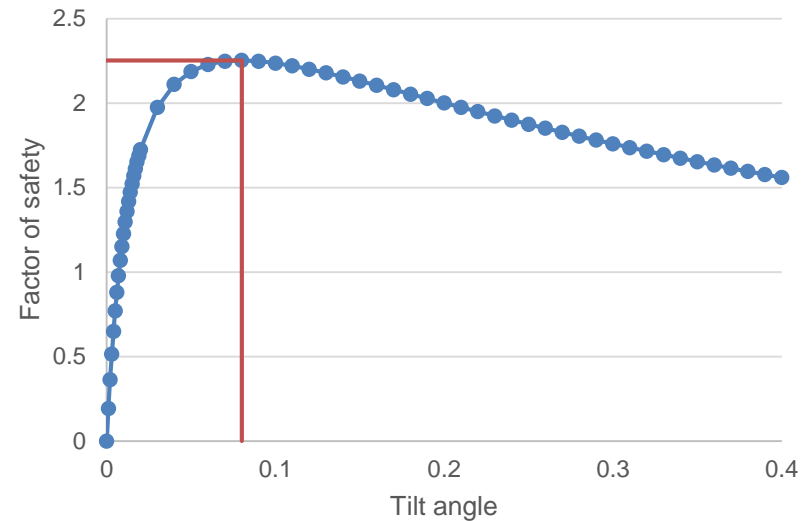
- Lateral deflection and rotation increases due to reduced post-cracking stiffness
- Ensure there is adequate lateral stiffness to avoid a lateral bending failure
- $EI_{eff} = \frac{EI_g}{1+2.5\theta}$
- Find tilt angle that maximizes the factor of safety

$$- \theta'_{max} = \sqrt{\frac{e_i}{2.5z_o}}$$

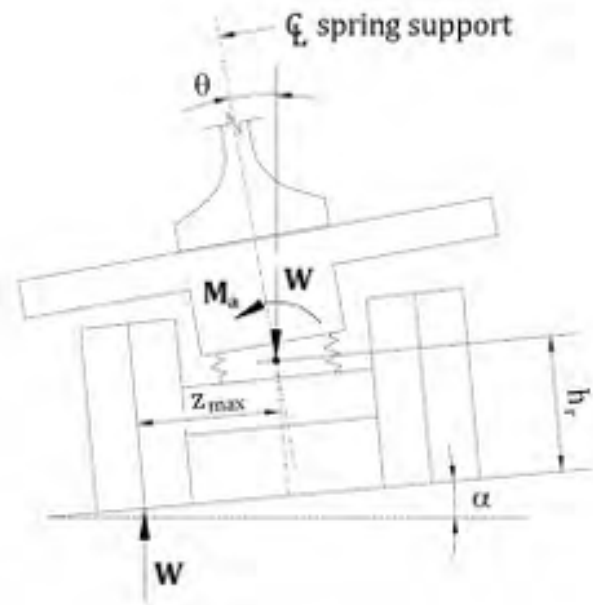
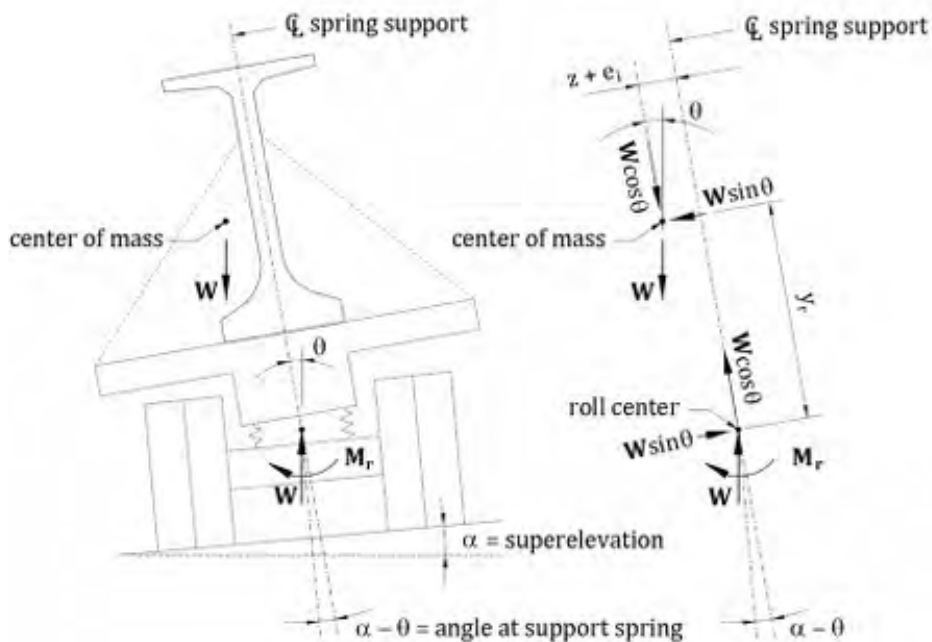
$$- M_{acting} = W(z_o\theta'_{max}(1 + 2.5\theta'_{max}) + e_i)$$

$$- M_{resisting} = W\theta'_{max}y_r$$

$$- FS' = \frac{M_{resisting}}{M_{acting}} \geq 1.5$$



Seated girders



Lateral bending

- Sum moments about roll center
- $M_{acting} = W((z + e_i) \cos \theta + y_r \sin \theta)$
- $M_{resisting} = K_\theta(\theta - \alpha)$
- $FS = \frac{M_{resisting}}{M_{acting}}$
- Stress, Cracking, Failure

Roll over stability

- Sum moments about roll center
- $M_{acting} = K_\theta(\theta - \alpha)$
- $M_{resisting} = W(z_{max} \cos \alpha - h_r \sin \alpha)$
- $FS = \frac{M_{resisting}}{M_{acting}}$

Hauling stability analysis

- Parameters: Girder stiffness, bunk locations, bunk eccentricity, superelevation, impact, **haul truck characteristics (wheel base and rotational stiffness)**, camber, sweep
- Case 1 – Normal crown slope – check stresses
 - 2% crown slope
 - $\pm 20\%$ vertical impact
- Case 2 – Max superelevation – check stresses and stability
 - 6% superelevation
 - No impact

Hauling stability analysis

- Ensure stresses are within acceptable limits
 - Tension: $0.0948\lambda\sqrt{f'_c}$
 $0.24\lambda\sqrt{f'_c}$ (with sufficient bonded reinforcement)
 - Compression: $0.65f'_c$ (plumb) $0.70f'_c$ (tilted)
- Ensure resisting moment exceed acting moments
 - Minimum FS against cracking = 1.0
 - Minimum FS against failure = 1.5
 - Minimum FS against roll over = 1.5

Improving factors of safety

$$FS = \frac{M_{resisting}}{M_{acting}}$$

- **Increase resisting moment**

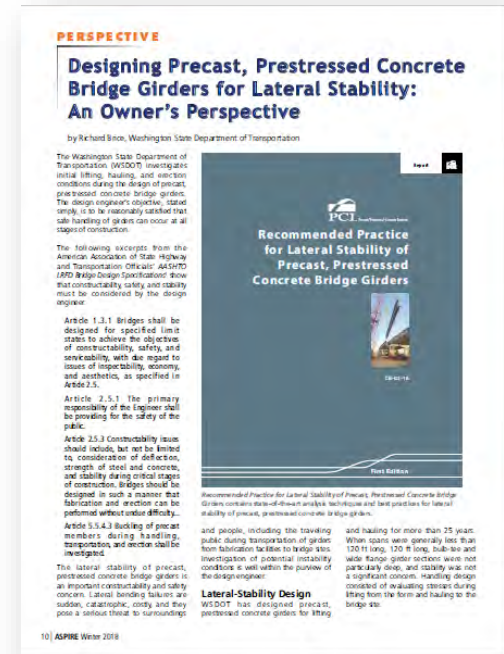
- Lifting – use rigid lift hardware to increase elevation of roll axis
- Hauling – use stiffer transport vehicle

- **Reduce acting moment**

- Reduce lateral deflection and rotation by increasing girder stiffness (f'_c, E_c) or providing lateral bracing
- Increase distance from end of girder to lift/bunk points
 - Reduces eccentricity from center of mass to roll center
 - Overhangs reduce mid-span lateral deflections
- *Reduces dead load moment that is balancing prestressing*
 - *Increases tension in top of girder*
 - **Temporary top strands** can mitigate excessive tensile stress

Stability design at WSDOT

- WSDOT design practices have adapted to changes in the precast industry
- See ASPIRE Magazine, Winter 2018 (Owner's Perspective)



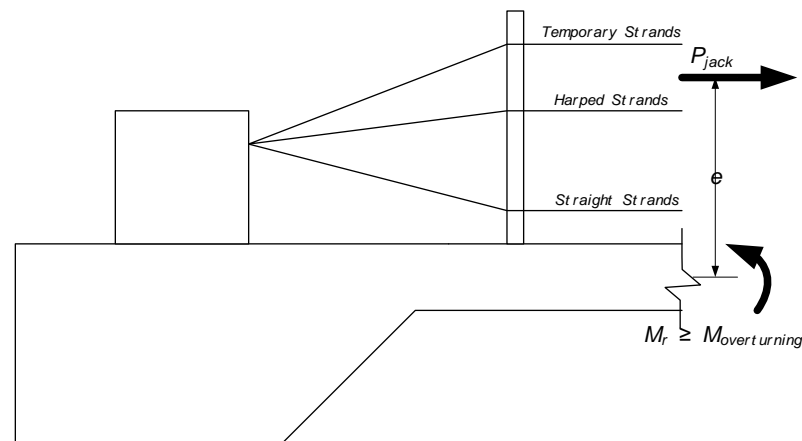
Goals

Engineer-of-record must be satisfied that girders can be safely fabricated, lifted, transported, and erected by available means and methods.

Avoid post-bidding design modifications.

Temporary top strands

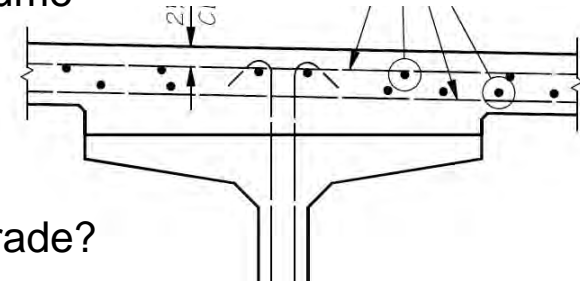
- (6) 0.6" ϕ strands are common
- Significant force at a high eccentricity above the stressing bed floor
- Tall girders, high prestressing, and TTS create fabrication challenges
- Girders need to be fabricated in existing facilities



TTS raise eccentricity and increase overturning moment on stressing bed

Post-design modifications

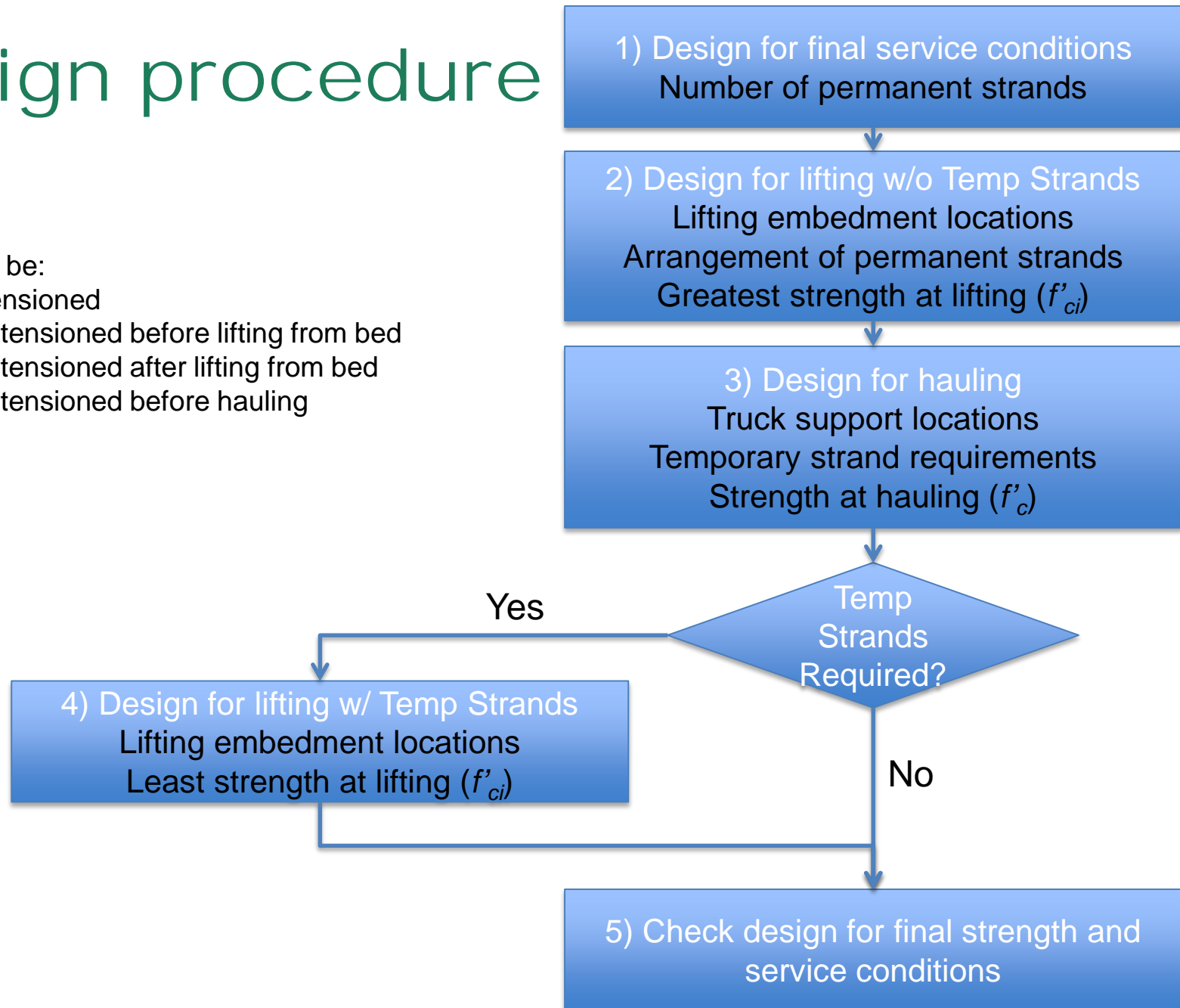
- **Design modifications after bidding are undesirable**
- Re-design required if capabilities of manufacturing facility are exceeded
- Changing prestressing arrangement (re-stranding and/or adding temporary top strands) effects initial concrete strength and camber
- Changes to camber effects haunch depth and concrete volume
 - This can be a significant quantity of material
 - Can the girders carry the extra dead load?
 - Who pays for the extra material?
 - What happens to bearing seat elevations and profile grade?



Design procedure

TTS can be:

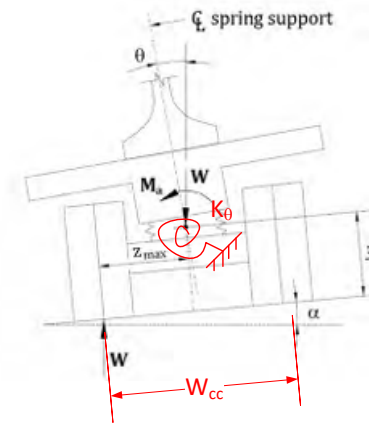
- pretensioned
- post-tensioned before lifting from bed
- post-tensioned after lifting from bed
- post-tensioned before hauling



Haul truck characteristics

- Design to least capable haul truck
- Use least value of K_{θ} from the table below along with the corresponding W_{cc} that provides adequate stability

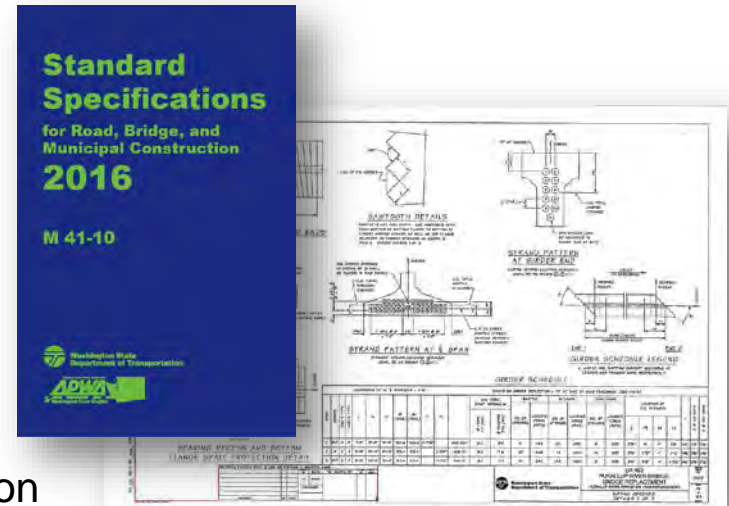
| K_{θ} (k-in/rad) | W_{cc} (in) |
|-------------------------|---------------|
| 40,000 | 72 |
| 50,000 | 72 |
| 60,000 | 72 |
| 60,000 | 96 |
| 70,000 | 96 |
| 80,000 | 96 |



- For Step 3 in design procedure, use parameters from row 1, then row 2, and so on, until stress and stability requirements are satisfied

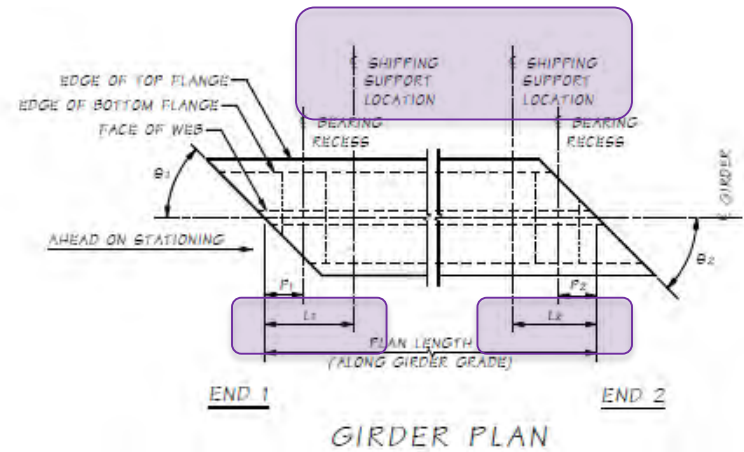
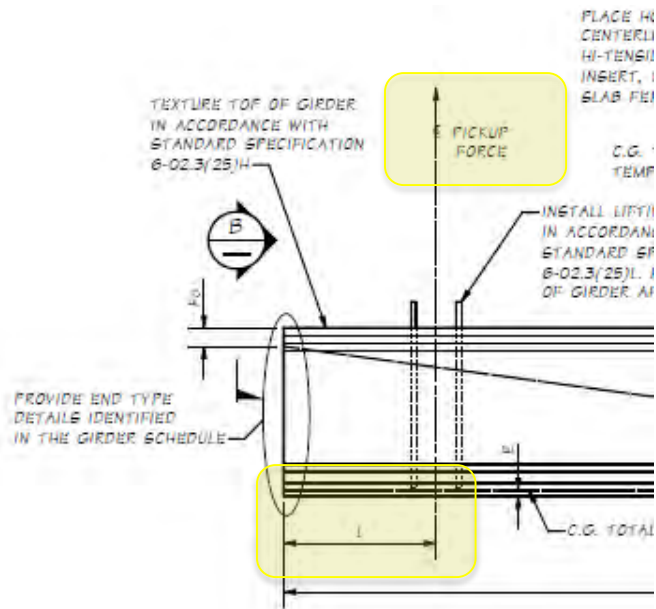
Communicating assumptions

- WSDOT Standard Specifications provide
 - allowable stress and stability requirements
 - assumed parameters
 - tolerances
- Contract girder schedule lists job specific information
- Contractor must submit PE stamped handling plans when deviating from the contract
 - Calculations must follow PCI recommendations



Job specific information

| SHIPPING AND HANDLING DETAILS | | | | | |
|---|---|----------------|----------------|----------------|-----------------|
| MAXIMUM MIDSPAN VERTICAL DEFLECTION AT SHIPPING | L | L ₁ | L ₂ | K _e | W _{cc} |
| | * | * | * | * | * |



Responsibility



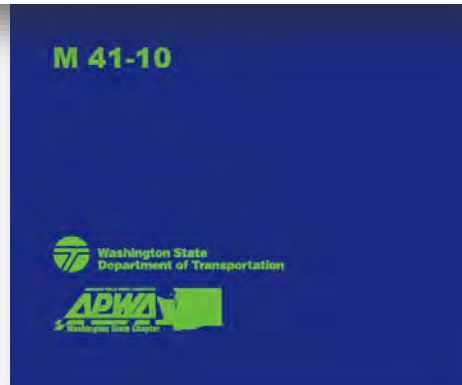
6-02.3(25)L1 Girder Lateral Stability and Stresses

The Contractor shall be responsible for safely lifting, storing, shipping and erecting prestressed concrete girders.

The Contract documents may provide shipping and handling details for girders including lifting embedment locations (L), shipping support locations (L_s and L_c), minimum shipping support rotational spring constant (K_r), minimum shipping support center-to-center wheel spacings (W_c), vertical deflection and number of temporary two strands. These shipping and handling details have been determined in accordance with Section 6-02.3(25)L2. The Contractor shall submit a Type 2E Working Drawing analyzing girder lateral stability.

6-02.3(25)L1 Girder Lateral Stability and Stresses

The Contractor shall be responsible for safely lifting, storing, shipping and erecting prestressed concrete girders.



The following design criteria shall be met:

1. Factor of Safety against cracking shall be at least 1.0
2. Factor of Safety against failure shall be at least 1.5
3. Factor of Safety against rollover shall be at least 1.5
4. Allowable concrete stresses shall be as specified in Section 6-02.3(25)L3

The analysis shall address any effects on girder vertical deflection (camber), "A" dimensions at centerline of bearings and deck spread cambers (C).

Shipping and handling details provided in the Contract documents have been determined using the following analysis assumptions:

1. Girder dimensions, strand locations and lifting embedment locations are within the tolerances specified in Section 6-02.3(25)I
2. Girder horizontal alignment (sweep) is within the tolerance specified in Section 6-02.3(25)I
3. Girder vertical deflection (camber) at midspan is less than or equal to the value shown in the Plans for shipping
4. Minimum concrete compressive strength at release (f'ci) has been reached before initial lifting from casting bed. Minimum concrete compressive strength at 28 days (f'c) has been reached before shipping
5. Height of girder bottom above roadway at shipping supports is less than or equal to 72 inches
6. Height of shipping support roll center above roadway is 24 inches ± 2 inches
7. Shipping support longitudinal placement (L_s and L_c) tolerance is ± 6 inches
8. Shipping support lateral placement tolerance is ± 1 inch
9. Shipping supports provide the minimum shipping support rotational spring constant (K_r) and minimum shipping support center-to-center wheel spacings (W_c) shown in the Plans

Designing for stability

- Designing for stability and optimized fabrication utilizes complex iterative analytical procedures
- Software can be used to quickly arrive at acceptable design solutions
- WSDOT design tools – part of the BridgeLink suite
 - PGSuper – precast/prestressed girder design with integrated stability analysis
 - PGSplice – precast spliced girder design with integrated stability analysis
 - PGStable – stand-alone girder stability analysis



<https://www.wsdot.wa.gov/eesc/bridge/software>

Additional Resources

- “Recommended Practice for Lateral Stability of Precast, Prestressed Concrete Bridge Girders”, PCI, CB-02-16
- PCI eLearning (coming soon)
 - Introductory Material
 - Stability of Hanging Girders
 - Stability of Transported Girders
 - Seated Girders and Stability Issues from Bed to Bridge
 - Stability Calculations and Sensitivity Analysis
- PCI Stability Spreadsheet (coming soon)
 - Detailed stability calculations
 - User guide



Questions?



Richard Brice, PE
Washington State Department of Transportation
Bridge and Structures Office
bricer@wsdot.wa.gov
360-705-7174