

**U-BEAM (TX-U54), SINGLE SPAN, PRECAST DECK PANELS, COMPOSITE DECK****9.6.12.3 Required Interface Shear Reinforcement/9.6.13 Minimum Longitudinal Reinforcement Requirement**

LRFD Eq. 5.6.4.1-3 can be solved for  $A_{vf}$  as follows:

$$7.98 = 0.28(31.5) + 1.0[A_{vf}(60) + 0]$$

Solving for  $A_{vf}$

$$A_{vf}(\text{req'd}) < 0.0 \text{ in.}^2/\text{ft}$$

Since the resistance provided by cohesion is greater than the applied force, provide the minimum required interface reinforcement.

**9.6.12.3.1 Minimum Interface Shear Reinforcement**

Minimum shear reinforcement,  $A_{vf} \geq (0.05A_{cv})/f_{yh}$

[LRFD Eq. 5.8.4.4-1]

From the design of vertical shear reinforcement, a No. 4 two leg bar at 10 in. spacing is provided from the beam extending into the deck. Therefore,  $A_{vf} = 0.48 \text{ in.}^2/\text{ft}$ .

$$A_{vf} = (0.48 \text{ in.}^2/\text{ft}) > (0.05 A_{cv})/f_{yh} = 0.05(31.5)/60 = 0.026 \text{ in.}^2/\text{in.} = 0.31 \text{ in.}^2/\text{ft} \quad \text{OK}$$

Consider further that LRFD Article 5.8.4.4 states that the minimum reinforcement requirement may be waived if  $v_{hi}/A_{cv} < 0.210 \text{ ksi}$  with surface roughened to an amplitude of 0.25 in.

$$7.18 \text{ kips/in.}/31.5 \text{ in.} = 0.228 \text{ ksi} > 0.210 \text{ ksi}$$

Therefore, the minimum reinforcement requirement cannot be waived.

**9.6.12.4 Maximum Nominal Shear Reinforcement**

$$V_{ni} \leq K_1 f'_c A_{cv} \text{ or } K_2 A_{cv}$$

$$V_n \text{ provided} = 0.28(31.5) + 1.0 \left( \frac{0.96}{12} (60) + 0 \right) = 13.62 \text{ kips/in.}$$

$$K_1 f'_c A_{cv} = (0.3)(4.0)(31.5) = 37.8 \text{ kips/in.}$$

$$K_2 A_{cv} = 1.8(31.5) = 56.7 \text{ kips/in.}$$

$$\text{Since provided } V_n \leq 0.3 f'_c A_{cv} \quad \text{OK}$$

[LRFD Eq. 5.8.4.1-4]

$$\leq 1.8 A_{cv} \quad \text{OK}$$

[LRFD Eq. 5.8.4.1-5]

**9.6.13 MINIMUM LONGITUDINAL REINFORCEMENT REQUIREMENT** [LRFD Art. 5.8.3.5]

Longitudinal reinforcement should be proportioned so that at each section, the following equation is satisfied:

$$A_{ps} f_{ps} + A_s f_y \geq \frac{M_u}{d_v \phi_f} + 0.5 \frac{N_u}{\phi_c} \left( \left| \frac{V_u}{\phi_v} - V_p \right| - 0.5 V_s \right) \cot \theta \quad \text{[LRFD Eq. 5.8.3.5-1]}$$

where

$A_{ps}$  = area of prestressing strand at the tension side of the section, in.<sup>2</sup>

$f_{ps}$  = average stress in prestressing strand at the time for which the nominal resistance is required, ksi

$A_s$  = area of nonprestressed tension reinforcement, in.<sup>2</sup>

$f_y$  = specified yield strength of reinforcing bars, ksi

$M_u$  = factored moment at the section corresponding to the factored shear force, ft-kips

$d_v$  = effective shear depth, in.

$\phi$  = resistance factor as appropriate for moment, shear and axial resistance. [LRFD Art. 5.5.4.2]  
Therefore, different  $\phi$  factors will be used for the terms in LRFD Equation 5.8.3.5-1, depending on the type of action being considered

$N_u$  = applied factored axial force, kips