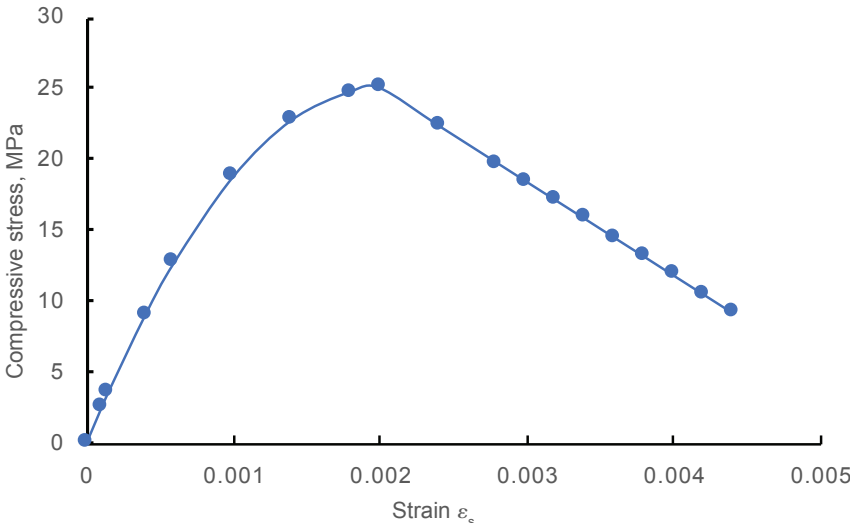


# Appendix: Numerical Modeling of Inverted-U-Shaped Connectors to Enhance the Performance of Composite Beams

Milad Khatib and Zaher Abou Saleh

This appendix contains additional tables and figures for “Numerical Modeling of Inverted-U-Shaped Connectors to Enhance the Performance of Composite Beams,” by Milad Khatib and Zaher Abou Saleh, which appears on pages 72–94 in the May–June 2023 issue of *PCI Journal*.

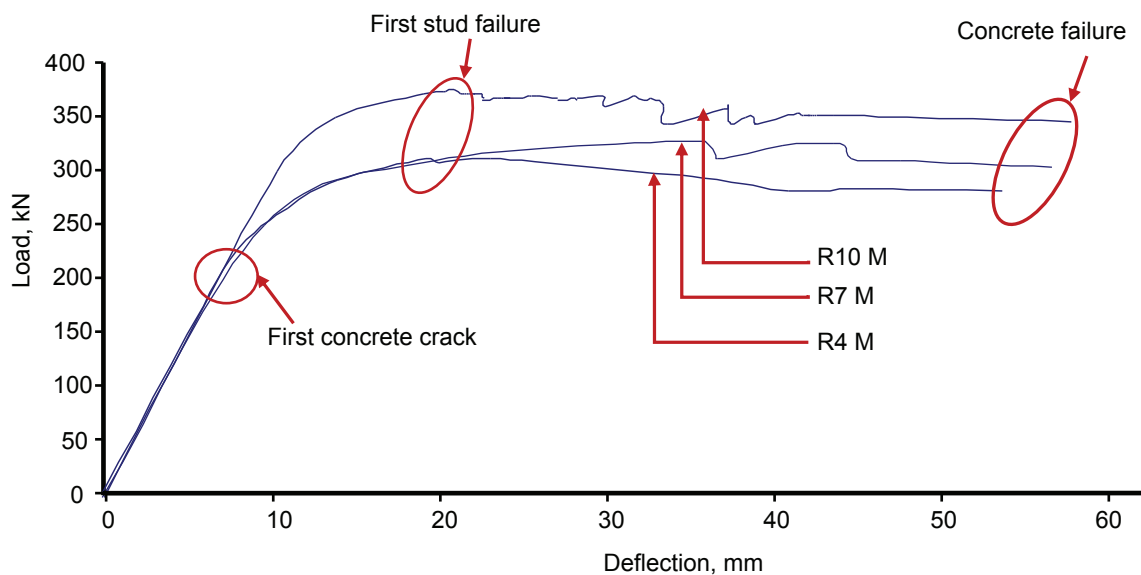
| Table A.1. Concrete properties for solid element model                                            |             |                                                                                                                                                                                                                                                   |
|---------------------------------------------------------------------------------------------------|-------------|---------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| Linear, elastic, isotropic                                                                        |             |                                                                                                                                                                                                                                                   |
| Modulus of elasticity<br>$E_c$ , MPa                                                              | 20,000      |                                                                                                                                                                                                                                                   |
| Poisson's ratio $\nu_c$                                                                           | 0.20        |                                                                                                                                                                                                                                                   |
| Nonlinear                                                                                         |             |                                                                                                                                                                                                                                                   |
| Strain                                                                                            | Stress, MPa |                                                                                                                                                                |
| 0                                                                                                 | 0           |                                                                                                                                                                                                                                                   |
| 0.0001                                                                                            | 2.4375      |                                                                                                                                                                                                                                                   |
| 0.00015                                                                                           | 3.6094      |                                                                                                                                                                                                                                                   |
| 0.0004                                                                                            | 9.0000      |                                                                                                                                                                                                                                                   |
| 0.0006                                                                                            | 12.7500     |                                                                                                                                                                                                                                                   |
| 0.00100                                                                                           | 18.7500     |                                                                                                                                                                                                                                                   |
| 0.00180                                                                                           | 24.7500     |                                                                                                                                                                                                                                                   |
| 0.00200                                                                                           | 25.0000     |                                                                                                                                                                                                                                                   |
| 0.00240                                                                                           | 22.3720     |                                                                                                                                                                                                                                                   |
| 0.00280                                                                                           | 19.7430     |                                                                                                                                                                                                                                                   |
| 0.00300                                                                                           | 18.4290     |                                                                                                                                                                                                                                                   |
| 0.00320                                                                                           | 17.1150     |                                                                                                                                                                                                                                                   |
| 0.00340                                                                                           | 15.8000     |                                                                                                                                                                                                                                                   |
| 0.00360                                                                                           | 14.4860     |                                                                                                                                                                                                                                                   |
| 0.00380                                                                                           | 13.1720     |                                                                                                                                                                                                                                                   |
| 0.00400                                                                                           | 11.8580     |                                                                                                                                                                                                                                                   |
| 0.00420                                                                                           | 10.5440     |                                                                                                                                                                                                                                                   |
| 0.00440                                                                                           | 9.2290      |                                                                                                                                                                                                                                                   |
| Nonlinear—inelastic—nonmetal plasticity                                                           |             |                                                                                                                                                                                                                                                   |
| Open shear transfer                                                                               | 0.3         | The value for each of these coefficients ranges between 0 and 1.0. The closed shear transfer coefficient value should be greater than the open shear transfer coefficient value. These two parameters determine the cracked concrete's stiffness. |
| Closed shear transfer                                                                             | 1.00        |                                                                                                                                                                                                                                                   |
| Uniaxial cracking stress, MPa                                                                     | 3.02        | $f_t = 0.30 \times 2/3$                                                                                                                                                                                                                           |
| Note: $f'_c$ = concrete compressive strength; $f_t$ = concrete tensile stress. 1 MPa = 0.145 ksi. |             |                                                                                                                                                                                                                                                   |

**Table A.2.** Material data for 140 mm deep steel I-beam (HEB 140)

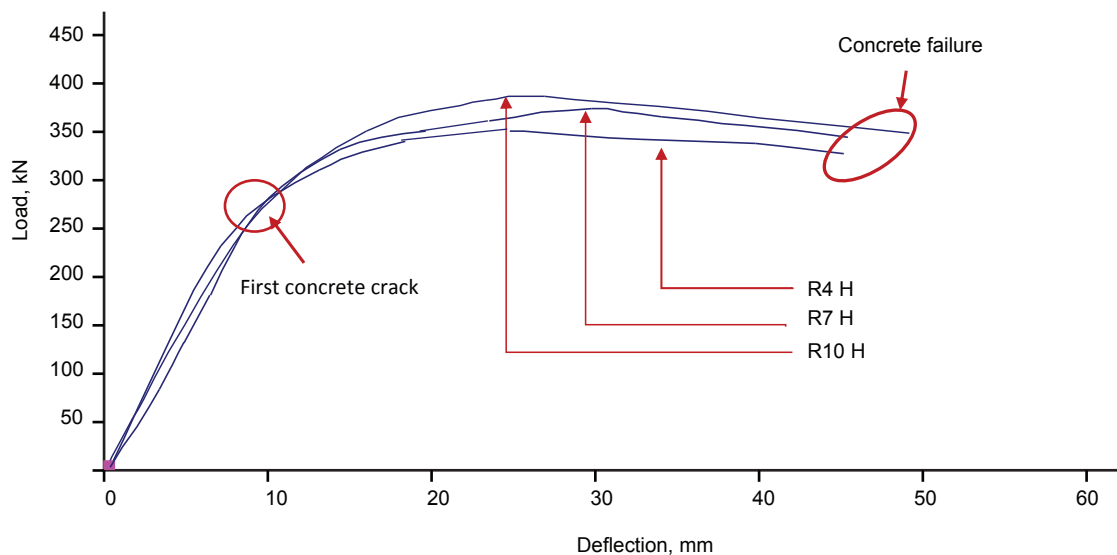
| Linear—elastic—isotropic                   |                        |
|--------------------------------------------|------------------------|
| Modulus of elasticity $E_s$                | $2.00 \times 10^5$ MPa |
| Poisson's ratio $\nu_s$                    | 0.3                    |
| Note: 1 mm = 0.039 in.; 1 MPa = 0.145 ksi. |                        |

**Table A.3.** Material data for steel reinforcement model

| Modulus of elasticity $E_s$    | $2.00 \times 10^5$ MPa |
|--------------------------------|------------------------|
| Poisson's ratio $\nu_s$        | 0.3                    |
| Nonlinear-inelastic-plasticity |                        |
| Yield stress $f_y$ (hard), MPa | 410                    |
| Yield stress $f_y$ (mild), MPa | 275                    |
| Note: 1 MPa = 0.145 ksi.       |                        |



**Figure A.1.** Experimental load deflection curves for mild (low-stiffness) stud connectors. Source: Reproduced with permission from Daou et al. (2021). Note: R4 M = specimen with four mild stud connector rows; R7 M = specimen with seven mild stud connector rows; R10 M = specimen with ten mild stud connector rows. 1 mm = 0.039 in.; 1 kN = 0.225 kip.



**Figure A.2.** Experimental load deflection curves for rigid (high-stiffness) stud connectors. Source: Reproduced with permission from Daou et al. (2021). Note: R4 H = specimen with four rigid stud connector rows; R7 H = specimen with seven rigid stud connector rows; R10 H = specimen with ten rigid stud connector rows. 1 mm = 0.039 in.; 1 kN = 0.225 kip.